

### **BOARD OF VARIANCE APPLICATION**

Date of Application: June 30, 2021 VOP File Number:										
Name: C0	ast Essen	tial Constr	ruction I	td	Postal Addre					1
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Phone: Ite		N		1 aman	g onaniu.					-
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	OWNER		HON:							
Name: 120	53735 BC	LID			Postal Addres	SS:				
Phone:					405	43 Thun	iderbird Ridg	e		-
Fax:					Squ	ıamish,	BC, V8B 0	G1		-
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PROPERTY I	NFORMAT	'ION:								
Civic Addres	S.			Legal De	escription: F	PID: 030	)-333-334, l	_ot 7, DL	. 211, EF	<u>'</u> S4695
7510 Pebble	Creek D	)rive		Zoning [	Designation:	RSA-2				_
Pemberton, E	3C			Section	in Bylaw to be	varied:	4.13 (a) vi	ii. & 7.21	l (a) i.	_
DESCRIPTIO	N OF VAR	IANCE RE	QUESTE	D:						
To vary Se	ction 4.13	3 (a) viii.	& Section	on 7.21 (	a) i. <mark>to</mark> allow f	or wall I	heights grea	ater than	1.2 m	_
to a maxim	um of 1.8	36 m, see	Site Pl	an A-01.	2, 2021-06-17	7				•
										•
APPLICATIO	N CHECKI	_IST:								
Certificate of	Fitle	Yes	🗌 No		Site Plan		Yes	No No	□ N/A	
Application Fe	e	V Yes	🗌 No		Property Within Land Reserve	n Agricult	tural 🗌 Yes	No	□ N/A	
Authorization	Form	Yes	🗌 No	□	Property Subje Riparian Area	ect to Regulatio	ons 🗌 Yes	V No	□	
Rationale for	√ariance	V Yes	🗌 No	□	Property Adjac Residential Pro	ent to operties	🛛 Yes	🗌 No	□ N/A	
Farhar I, Coast	ig Shahidi Essential C	onstruction	n Ltd	here	by allow for the	e purpos	ses of this ap	plication,	any	
member(s) of the Board of Variance to vie										
For Office Use Only										
Roll No.:					Prospero No.:	:				
Related File	es:				Fee Submitte	d: <u>\$</u>	Receip	ot No.:		

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### **Letter of Agency**

Civic Address: 7510 Pebble Creek Drive, Pemberton, BC

PID#:030-333-334

Legal Description: LOT 7 PLAN EPS4695 DISTRICT LOT 211 LILOOET LAND DISTRICT

(the "Subject Lands")

Registered Owner: 1283735 BC LTD

I, <u>Rachael Marcisla</u>, being the Registered Owner (or duly authorized representative of the Registered Owner) of the Subject Lands, hereby authorize <u>Coast Essential Construction Ltd</u> to act as Agent and authorized signatory for the Registered Owner in respect of all matters relating to the development application of the Subject Lands as may be required by the Village of Pemberton.



June 16 2021

Date

File Reference: Declared Value \$540000

#### \*\*CURRENT INFORMATION ONLY - NO CANCELLED INFORMATION SHOWN\*\*

Title Issued Under	STRATA PROPERTY ACT (Section 249)			
Land Title District Land Title Office	KAMLOOPS KAMLOOPS			
Title Number From Title Number	CA8777474 CA6601994			
Application Received	2021-02-17			
Application Entered	2021-02-19			
Registered Owner in Fee Simple Registered Owner/Mailing Address:	1283735 B.C. LTD., INC.NO. BC1283735 BOX 2563 GARIBALDI HIGHLANDS, BC V0N 1T0			
Taxation Authority	North Shore - Squamish Valley Assessment Area Pemberton, Village of Pemberton Valley Dyking District			
Description of Land Parcel Identifier: Legal Description: STRATA LOT 7 DISTRICT LOT 211 TOGETHER WITH AN INTEREST IN ENTITLEMENT OF THE STRATA LO	030-333-334 LILLOOET DISTRICT STRATA PLAN EPS4695 N THE COMMON PROPERTY IN PROPORTION TO THE UNIT OT AS SHOWN ON FORM V			
Legal Notations HERETO IS ANNEXED EASEMENT CA2874965 OVER LOT 2, PLAN EPP21848 AS TO PART FORMERLY LOT 3 PLAN EPP21848				

HERETO IS ANNEXED EASEMENT CA6555926 OVER THAT PART OF STRATA LOT 6 LD STRATA PLAN EPS4695 SHOWN ON PL EPP78129

#### TITLE SEARCH PRINT

File Reference: Declared Value \$540000

**Charges, Liens and Interests** 

Nature: Registration Number: Registration Date and Time: Remarks:

Nature: Registration Number: Registration Date and Time: Registered Owner: Remarks:

Nature: Registration Number: Registration Date and Time: Registered Owner: Remarks:

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Nature: Registration Number: Registration Date and Time: Registered Owner: Remarks:

Nature: Registration Number: Registration Date and Time: Registered Owner: Remarks:

Nature: Registration Number: Registration Date and Time: Registered Owner: Remarks: EASEMENT CA2874965 2012-11-19 15:13 INTER ALIA APPURTENANT TO LOT 3, PLAN EPP21848, AS TO PART FORMERLY LOT 2 PLAN EPP21848

COVENANT CA4950098 2016-01-26 17:04 VILLAGE OF PEMBERTON INTER ALIA MODIFIED BY CA7195407

STATUTORY RIGHT OF WAY CA5871774 2017-03-15 10:06 BRITISH COLUMBIA HYDRO AND POWER AUTHORITY INTER ALIA AS TO PART FORMERLY LOT 2 PLAN EPP21848

STATUTORY RIGHT OF WAY CA5871775 2017-03-15 10:06 TELUS COMMUNICATIONS INC. INTER ALIA AS TO PART FORMERLY LOT 2 PLAN EPP21848

COVENANT CA6513043 2017-12-14 15:57 VILLAGE OF PEMBERTON INTER ALIA

COVENANT CA6513049 2017-12-14 15:57 VILLAGE OF PEMBERTON INTER ALIA

COVENANT CA6513055 2017-12-14 15:57 VILLAGE OF PEMBERTON INTER ALIA

#### TITLE SEARCH PRINT

File Reference: Declared Value \$540000

> Nature: Registration Number: Registration Date and Time: Remarks:

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> Nature: Registration Number: Registration Date and Time: Remarks:

Nature: Registration Number: Registration Date and Time: Registered Owner:

**Registered Owner:** 

Remarks:

Nature: Registration Number: Registration Date and Time: Registered Owner:

**Registered Owner:** 

Remarks:

RESTRICTIVE COVENANT CA6513056 2017-12-14 15:57 INTER ALIA APPURTENANT TO PCL A ( DD W34182F PL A21) DL 211 LILLOOET DISTRICT

STATUTORY BUILDING SCHEME CA6555908 2018-01-09 14:58 INTER ALIA

EASEMENT CA6555917 2018-01-09 14:58 INTER ALIA PART IN PLAN EPP78097 APPURTENANT TO THE COMMON PROPERTY STRATA PLAN EPS4695

EASEMENT CA6555927 2018-01-09 14:58 PART IN PLAN EPP78129 APPURTENANT TO STRATA LOT 6 LD STRATA PL EPS4695

MORTGAGE CA7093931 2018-09-27 16:37 379489 ONTARIO STREET HOLDINGS LTD. INCORPORATION NO. C0436510 AS TO AN UNDIVIDED 400/650 INTEREST HARRY BING PARK JUNG LINDA GAIL JUNG AS TO AN UNDIVIDED 250/650 INTEREST AS JOINT TENANTS INTER ALIA

ASSIGNMENT OF RENTS CA7093932 2018-09-27 16:37 379489 ONTARIO STREET HOLDINGS LTD. INCORPORATION NO. C0436510 AS TO AN UNDIVIDED 400/650 INTEREST HARRY BING PARK JUNG LINDA GAIL JUNG AS TO AN UNDIVIDED 250/650 INTEREST AS JOINT TENANTS INTER ALIA

<b>TITLE SEARCH PRINT</b> File Reference: Declared Value \$540000		2021-06-17, 14:55:48 Requestor: Martha McLellan
Duplicate Indefeasible Title	NONE OUTSTANDING	
Transfers	NONE	
Pending Applications	NONE	





604-366-8116 info@coastessential.com coastessential.com

#### Letter to the Board of Variance: 7510 Pebble Creek Drive, Lot 7, The Ridge

June 30, 2021

Village of Pemberton Box 100, 7400 Prospect Street Pemberton, BC, V0N2L0

Dear Board of Variance,

This letter outlines our rationale and justification for the variance requests outlined below for the proposed residential construction at 7510 Pebble Creek Drive. This permit would cover the construction of a series of landscape reatining walls at the southern edge of the property taller than those currently permitted by Sections 4.13 and 7.21 of Village Zoning Bylaw No.832, 2018. With our intent being a design solution that we feel is preferable to a bylaw compliant alternative.

Section 4.13 (a) viii. allows a retaining wall to a maximum height of 1.2 meters (m) to be sited on any portion of a lot, and Section 7.21 (a) i. allows a retaining wall up to 1.2 m in height when measured from the average natural grade at it's base, and not within 0.6 m horizontally of any another retaining wall.

The following variances are being requested on the walls denoted on the plan and section drawings, and the table below:

- To vary Section 4.13 (a) viii. to allow for wall heights greater than the 1.2 m (3.94 ft) allowed, to a maximum of 2.03 m (6.67 ft), a maximum relaxation of 0.82 m (2.67 ft), to be sited on the lot in general complicance with location on the Site Plan A-01.2 and Site Section A-01.6 2021-06-17 or in a location approved by Building Permit; and
- To vary Section 7.21 (a) i. to relax the maximum height of a retaining wall, from 1.2 m (3.94 ft) to maximum of 2.03 m (6.67 ft), a maximum relaxation of 0.82 m (2.67 ft).

	Maximum Height from Average Natural Grade	Relaxation Requested	Maximum Exposed Face of Wall Showing on Completion
Wall 1 (Yellow)	1.52 m (5.00 ft)	0.30 m (1.00 ft)	1.52 m (5.00 ft)
Wall 2 ( <mark>Blue</mark> )	1.73 m (5.67 ft)	0.51 m (1.67 ft)	1.98 m (6.50 ft)
Wall 3 (Purple)	1.50 m (4.92 ft)	0.28 m (0.92 ft)	1.98 m (6.50 ft)
Wall 4 (Green)	2.03 m (6.8 ft)	0.82 m (2.67 ft)	1.73 m (5.67 ft)



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The proposed walls have been designed in conjunction with input from our geotechnical engineers for the project, their independent review and recommendations for our design are submitted alongside this request, and designed in a manner that we feel matches the existing contours of the lot more so than would have been possible when constructing a bylaw compliant arrangement.

We feel the plan presented provides an aesthetically suitable solution which matches the natural conturs of the landscpae and is inline with the Village of Pemberton's Hillside Development Design Guidelines (April 2020). Whilst the current bylaws, which allow for a single retaining wall to a maximum of 1.2 m in height, not located within 0.6 m horizontally of any other retaining wall, or 1V:0.5H, presents design constraints which make the creation of retaining walls matching natural conturs difficult to achieve without arriving at a staircase-like solution.

Additionally, our submitted plan creates a greater amount of usable property in the form of a flatter, less tiered, landscpae design which improves accessibility around the proposed dwelling. It also allows for efficient and effective distribution of existing fill material within the lot, reducing the need for removal of spoil during excation and importing of replacement fill for backfilling later on, both of which minimising the project's need for heavy vehicle traffic.

In conjunction with this letter and our appilcation we have also submitted the following information in relation to this variance request:

- A completed application form
- A site plan
- Retaining wall design drawings
- Retaining wall and landscape plan visual renderings
- Geotehcnical review and design guidance
- A landscape plan
- A landscape cost estimate
- Certificate of title (2021-06-07)
- Owners Authorization Form
- Site photos (included below)

As a reference for council, we are also submitting a similar development variance permit request for the adjacent property, 7508 Pebble Creek Drive, which shares an owner with this lot. The two properties will employ a similar aesthetic in their landscape retaining design, with the extent of retaining varying to account for the individual lot characteristics.



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Figure 1 – Current conditions at southern property line looking east.



Figure 2 – Current conditions at southern property line looking north.



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Figure 3 – Current conditions at southern property line looking west.



Figure 4 – Current conditions facing south from lot center, existing neighbouring retaining visible.



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Figure 5 – Current view southwest from within the subject lot, showing extent of neighbouring retaining. This example of existing retaining does not meet current bylaws.



Figure 6 – Center lot facing west showing relative scale of the neighbouring property's retaining. Example of existing wall more than 1.2m in height.



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Figure 7 – Local retaining wall example 1 of 6. Example of well executed terraced walls. Location: The Ridge.



Figure 8 – Local retaining wall example 2 of 6. Walls over height but including terrace for planting. Location: main access to The Ridge.



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Figure 9 – Local retaining wall example 3 of 6. Example of how each row of gabions step back to easily achieve a 1H:1V slope. Location: main access to The Ridge.



Figure 10 – Local retaining wall example 4 of 6. Example of 1H:IV slope. Although considered acceptable, this solution is not aesthetically pleasing, and prone to erosion. Terraced retaining provides better vegetation cover, less erosion, and a more pleasing appearance. Viewed from Sunstone Development.



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Figure 11 – Local retaining wall example 5 of 6. Example of an unretained slope. Viewed from Sunstone Development.



Figure 12 – Local retaining wall example 6 of 6. Developer built wall in Sunstone for road access. As all hillside developments will require road access, so do homes. Lots sloping in two directions require retaining across the slopes from a high wall to a low wall in order to use the lot efficiently.

Page 8 of 8



200-100 Park Royal West Vancouver BC V7TIA2 604 366 8116 reid@coastessential.com coastessential.com

#### 7510 Pebble Creek Drive Lot 7, The Ridge

#### Landscape Plan and Cost Estimate

June 15, 2021

Village of Pemberton Box 100, 7400 Prospect Street Pemberton, BC, V0N2L0

Dear Development Services Team,

The following plant list is the planned landscape planting scheme outlined on the design drawings, and estiamted to cost approximately \$6,500 plus taxes at current prices.

#### Trees:

•	Pinus Nigra	1.5 m to 2 m	Quantity 9			
•	Pacific Dogwood	5 cm cal.	Quantity 3			
•	Quaking Aspen	5 cm cal.	Quantity 1			
•	Multi stem Vine Maple	2 m	Quantity 1			
Shrub	s:					
•	Mugo Pines	#2 pot	Quantity 9			
٠	Yellow Twig Dogwood	#2 pot	Quantity 14			
٠	Nootka Rose	#2 pot	Quantity 9			
٠	Mock Orange	#5 pot	Quantity 1			
•	Oregon Grape (Mahonia Nervosa)	#2 pot	Quantity 12			
Groun	dcover					
•	Kinnickinnick	4" pot	Quantity 144			
Softscape:						

• To include all required top soils and mulch

#### **Estimated Cost:**

• \$6,500 plus taxes at current prices.

PREPARED FOR: COAST ESSENTIAL CONSTRUCTION

# 7510 PEBBLE CREEK DRIVE (LOT 7)

## **RETAINING WALL DESIGN**

PEMBERTON, BC



SFA GEOTECHNICAL INC #1 - 38920 Queens Way Squamish, BC V8B 0K8 604 898 1093

### **ISSUED FOR DISCUSSION**

JOB NO:	1547
Drawing No:	1547-7-00
DATE:	MAY 25, 2021





#### SECTION 1

		THE RIDGE, LOT 7	DRAWN BY	WG	REVISION N	10.:
SFA	SFA GEOTECHNICAL INC	PEMBERTON BC	REVIEWED BY	SE	Α	ISSUED
	Squamish, BC V8B 0K8		DATE MA	Y 14 2021		
GEOTECHNICAL	604 898 1093		SCALE:	NTS		
ENGINEERING		SECTION VIEW	SOALL.	NIS		

1.6 m			
			890
			888
PROPOSED	GRADE		
			886
NG GRADE			884
			882
			880
			878
			876
			074
			874
			872
			970
			868
			866
			864
			862
FOR DISCUSSION MAY 25, 202	JOB NO:	1547	
	DRAWING NO.		
		1547-7-02	2



SECTION 2

		THE RIDGE, LOT 7	DRAWN BY	,	WG	REVISION NO .:	DESCRIPTION	DATE		1 5 1 7
SFA	SFA GEOTECHNICAL INC #1 - 38920 Queens Way	PEMBERTON, BC	REVIEWED	BY:	SF	A	ISSUED FOR DISCUSSION	MAY 25, 2021	JOB NO:	1547
	Squamish, BC V8B 0K8	RETAINING WALL	DATE:	MAY 14, 2	2021				DRAWING NO:	
ENGINEERING	604 898 1093	SECTION VIEW	SCALE:	١	NTS					1547-7-02

888
886
 884
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 872

890



#### SECTION 3

0.5.4		THE RIDGE, LOT 7	DRAWN BY: WG	REVISION NO .:	DESCRIPTION	DATE	100 110	1547
SEAL	#1 - 38920 Queens Way	PEMBERTON, BC	REVIEWED BY: SF	A	ISSUED FOR DISCUSSION	MAY 25, 2021	JOB NO:	10-11
0171	Squamish, BC V8B 0K8	RETAINING WALL	DATE: MAY 14, 2021				DRAWING NO.	
ENGINEERING	604 898 1093	SECTION VIEW	SCALE: NTS				bivillito no.	1547-7-04

8.5 m —	
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PROPOSED GRADE	888
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**TYPICAL SECTION** 

SCALE:

NTS

604 898 1093

PRIMARY REINFORCEMENT LENGTH AS SHOWN ON SECTIONS

REINFORCED BACKFILL

DESCRIPTION	DATE		1617
FOR DISCUSSION	MAY 25, 2021	JOB NO:	1547
		DRAWING NO:	
			1547-7-05





NOTES:

1. SEE WELDED WIRE FORM (WWF) FACING DETAIL AND WWF OUTSIDE CORNER UNIT DETAIL FOR FACING MATERIALS AND DIMENSIONS.

2. INSTALL ADJACENT WWF FACING UNITS TO PROVIDE 100mm OVERLAP OF HORIZONTAL WIRES.

TOP OF WWF WALL, FINISHING DETAIL



2. 3. TERMINATE GEOGRIDS NO MORE THAN 75mm FROM PIPE.

CONTRACTOR RESPONSIBLE TO INSTALL PIPE WITH LEAK-PROOF JOINTS. 4

PIPE PENETRATION DETAIL AT WWF WALL FACE

		THE RIDGE, LOT 7	DRAWN BY	WG	REVISION NO.	DESCRIPTION	DATE		1517
	SFA GEOTECHNICAL INC	PEMBERTON BC	REVIEWED BY	SE	A	ISSUED FOR DISCUSSION	MAY 25, 2021	JOB NO:	1547
	Squamish BC V8B 0K8	RETAINING WALL		V 1/1 2021					
GEOTECHNICAL	604 898 1093			NTO				DRAWING NO:	1517 7 07
ENGINEERING		TYPICAL DETAILS (2/2)	JUALE.	NIS					1347-7-07

NOTES:

-SELECT FILL (TYPE AND LIMITS BY OTHERS)

REINFORCED FILL

CONTINUE FACING COMPONENTS FROM FACING UNIT ALONG PIPE

#### 1. GENERAL

- 1.1. In these Notes, the Engineer is SFA Geotechnical.
- 1.2. These notes must be read in conjunction with 1547-7-01 to 1547-7-07.
- The work described and shown involves the supply and installation of geogrid reinforced retaining walls with NILEX SIERRA SLOPE.
- 1.4. The retaining wall will be installed on an excavated, natural, undisturbed subgrade, or approved subgrade fill at the locations shown in the Architectural Drawings.
- 1.5. The Contractor shall confirm the locations and conditions of all man-made structures which could be affected or damaged by the work. Structures which may be affected or damaged by the work must be reported to the Engineer in advance of the work to take place. The Engineer may change the design or approve of modifications to installation techniques proposed by the Contractor to preclude damage or conflict with existing structures.
- 2. MATERIALS
- 2.1. The proposed retaining wall have been designed on the basis of Nilex Sierra Slope retaining walls inclined at 3H:8V as indicated in the design drawings. Alternate methods of support system will require redesign of the walls by the Engineer and may not be substituted without written authorization from the Engineer.
- 2.2. GEOGRID The retaining walls have been designed on the basis of Tensar UX1500 with a long term design strength (a maximum allowable design strength of 52 kN/m for an 120 year design life. Alternative geogrid will require a redesign of the wall by the Engineer and may not be substituted without written authorization of the engineer. Geogrid coverage shall be 100%.
- 2.3. LEVELING PAD Leveling pad fill shall consist of at least 300 mm of 19 mm minus crushed gravel.
- 2.4. FOUNDATION SOIL FOUNDATION SOIL shall consist of native soils approved by the Engineer. Any grade reinstatement of the subgrade shall be completed using leveling pad materials.
- 2.5. REINFORCED BACKFILL REINFORCED BACKFILL should consist of 75 mm minus pit run sand and gravel containing less then 2% fines or material otherwise approved by SFA.
- 2.6. RETAINED BACKFILL RETAINED backfill should consist of clean sand or sand and gravel fill, with less than 5% fines.

#### 3. EXECUTION

**RETAINING WALL** 

NOTES

3.1. The native soils shall be sloped at MAX 3H:4V. The cut slopes

	may need to be flatter to satisfy soil conditions encounter	red.			1.0
3.2.	The Engineer will inspect the excavation and approve su prior to the placement of any fill soils.	lbgrade		3.15	Trac upor
3.3.	The first course of the facing units shall be placed on the subgrade and alignment and level checked.	e approved			geog prev
3.4.	The reinforced backfill shall be placed and compacted be facing units and to the extent to the reinforced backfill sh the cross sections.	ehind the Iown in		4.	
3.5.	Where the geogrid reinforcement is required, the geogrid reinforcement shall be placed to the facing units as per the manufactures instructions. The length and spacing of the reinforcement is shown on the cross sections.	d he ∋ geogrid			The hour of th
3.6.	Minimum burial depth shall be achieved after the first cou facing units is completed.	urse of			<ul> <li>Site</li> <li>Plac</li> <li>Drai</li> <li>behi</li> </ul>
3.7.	The geogrid reinforcement shall be placed at the elevation the extent shown on the cross sections or as directed by Engineer.	ons and to the			• Geo
3.8.	The geogrid shall be laid horizontally in the direction perp to the face of the retaining wall. The geogrid shall be pull of wrinkles and anchored prior to the backfill placement of geogrid.	pendicular I taut, free on the			
3.9.	The geogrid reinforcement shall be continuous (COVER/ RATIO OF 100%) throughout the embedment lengths wi overlap. Spliced connections between shorter pieces of g are not permitted.	AGE th no geogrid			
3.10	. Where georid layers overlap a minimum of 75 mm of gra be placed between the layers.	vel should			
3.11	<ul> <li>Reinforced backfill shall be placed, spread, and compact such a manner that minimizes the development of slack geogrid.</li> </ul>	ed in in the			
3.12	. Reinforced and retained backfill shall be placed and com lifts not to exceed 300 mm.	pacted in			
3.13	. Reinforced and retained backfill shall be compacted to 9 maximum density as determined by ASTM 1557 (Modifie Proctor) or equivalent. The moisture content of the backf prior to and during compaction shall be uniformly distribut throughout each layer and shall be within 2% of the optim moisture content for compaction.	5% of the ed ill material ited num			
THE RI	DGE, LOT 7	DRAWN BY:	WG	REVISION NO	
PEMBE	RTON, BC	REVIEWED BY:	SF	A	1550



DRAWN BY	WG	REVISION NO .:	DESCRIPTION	DATE		1517
REVIEWED BY	SE	Α	ISSUED FOR DISCUSSION	MAY 25, 2021	JOB NO:	1547
	2021					
	NTO				DRAWING NO:	1617 7 09
SCALE.	NIS					1547-7-00

3.14. Only lightweight hand-operated equipment shall be allowed within 1.0 m of the front face of the facing units.

icked construction equipment shall not be operated directly on the geogrid reinforcement. A minimum fill thickness of 150 is required prior to operation of tracked vehicles over the ogrid. Track vehicles should not turn while on the geogrid to vent tracks from displacing the fill and geogrid and damage or ck to result in the geogrid.

#### **NSTRUCTION INSPECTION**

e Contractor shall notify SFA Geotechnical Inc. a minimum 48 irs in advance of the commencement of the following aspects he work:

Stripping & Foundation Excavation

cement of the initial course of the facing units

in Pipe Placement & Backfill Placement and Compaction

ind wall

ogrid Placement



1 – 38920 Queens Way Squamish, BC V8B 0K8 604-898-1093

Coast Essential Construction Ltd. 200 – 100 Park Royal West Vancouver, BC V7T 1A2 March 5, 2021 File: 1547

Attention: Mr. Reid Madiuk

#### Re: Preliminary Geotechnical Report, Proposed New Home, Lot 7, The Ridge, Pemberton, BC

#### **1.0 INTRODUCTION**

It is proposed to construct a new home on Lot 7 of The Ridge subdivision in Pemberton. The lot has legal description: Strata Lot 7 District Lot 211 Lillooet District Strata Plan EPS4695. SFA Geotechnical Inc. (SFA) has been engaged to provide geotechnical recommendations for the project.

The concept of the new home is in its preliminary stages and therefore drawings are not yet available. We understand that the home will be two to three stories and that a walk out basement is being considered. The home is to be constructed over reinforced concrete foundations with concrete foundation walls and wood frame construction above.

We understand that several retaining walls are to be included in the final design of the property and that a pool may be considered.

This report presents the results from our test pit investigation completed on February 24, 2021 and provides preliminary geotechnical recommendations for the proposed home. Additional geotechnical recommendations may be required for the retaining walls and the pool once the project scope has been confirmed.

This report has been prepared exclusively for our client, for their use, and the use of others on their design team, however, it remains the property of SFA Geotechnical Inc.

#### **2.0 SITE DESCRIPTION**

The lot is the  $7^{th}$  lot on the south side of the road accessing the subdivision. The property is bound to the north by the subdivision access road, to the south by a right of way, to the east by a private residential property and to the west by an undeveloped lot. The lot is undeveloped and generally slopes down to the south southwest.

A rock stack retaining wall has been constructed where the lot meets the subdivision access road to create a driveway for site access. The rock stack wall is aligned along the west property line. The lot has been recently filled with dredged river sand and gravel fill to create a level bench across the lot. The site preparation efforts and placement schedule of the fill are unknown.

#### **3.0 FIELD INVESTIGATION**

SFA visited the site on February 24, 2021 to complete a preliminary test pit investigation. Four shallow test pits were completed linearly down the fill slope to help characterize the native subgrade, determine the site preparation efforts and placement schedule of the fill, and determine the fill thickness in the area of the proposed home.

#### 4.0 SUBSURFACE CONDITIONS

#### 4.1 Soil Conditions

In general, the soil profile noted from the surface downwards consists of sand and gravel fill over topsoil, underlain by topsoil and weathered glacial till and in turn dense glacial till.

#### Fill (Dredged River Sand and Gravel)

The dredged river sand and gravel is generally clean and well graded between medium grained sand and medium grained gravel. The particles are rounded and loosely packed. The fill is at least 3.5 m thick at the crest of the fill slope.

#### Topsoil

The fill is underlain by approximately 0.3 m to 0.45 m of organic topsoil. The topsoil contains roots up to 2 cm thick and is dark brown to black.

#### Glacial Till

The fill is underlain by glacial till which comprises poorly graded silty sand and gravel. The upper 0.5 m of the glacial till is weathered and loose. The glacial till becomes dense at approximately 1 m below the pre-filling ground surface where it grades from tan to grey in colour.

For a more detailed description of the subsurface conditions refer to the test hole logs in Appendix A.

#### 4.2 Groundwater Conditions

Perched ground water was observed within the glacial till. Based on surrounding grades it is likely that perched water is present within this stratum during wetter months and following snow melt. The perched water flow into the test pits was minor.

#### **5.0 DISCUSSION**

#### **5.1 General Comments**

SFA visited the site on February 24, 2021 to review four shallow test pits within the building envelope. The test pits indicate that the subgrade was not stripped of organics and that the fill appeared to be bulked into the site with no form of formal compaction.

Tension cracks were observed approximately 2 metres back from the crest of the recently placed sand and gravel slope. The slope below is at approximately 45 to 50 degrees which is steeper than would be recommended for this material type. The fill is not considered to be seismically stable.

The lot is accessed by a gravel driveway supported by a rock stack wall which we understand was constructed as part of the subdivision development. The rock stack wall ranges in height from approximately 2 m to 3.5 m and has a batter of approximately 1H:4V (horizontal to vertical). There is no evidence of geogrid or any other form of internal support for the wall.

SFA was not involved with the construction of the retaining wall or placement of fill and therefore cannot confirm how the retaining wall was constructed or how the fill was placed.

Provided the geotechnical consideration above are addressed as described below, we are of the opinion that the project is feasible from a geotechnical standpoint.

#### 5.2 Site Grading Fill

The recently placed site grading fill is loose and was placed over organics. We understand that a project goal is to keep as much of the fill on site as possible. Therefore, it will be required to move the fill around the lot during the stripping of organics and construction of the home.

The fill slopes are not stable. Tension cracks indicate that the slope is moving, and failure could occur if the fill becomes saturated. If the face of the fill slope were to slide, the toe debris could extend beyond the property lines. We recommend that temporary fill slopes be reduced to 2.5H:1V to reduce the risk of slope failure.

Retaining walls will likely be required to achieved desired site grades and to accommodate the re-use of the fill which has been placed on the site. We estimate that the retaining walls will exceed 1.2 m in height and therefore will need to be engineered and a variance will be required from the Village of Pemberton in advance of their construction.

If it is decided that engineered retaining walls are required SFA would be able to provide retaining wall design and drawings once final site grading is known.

#### **5.3 Existing Retaining Wall**

The design and construction of the rock stack retaining wall which supports the gravel site access is not known. The retaining wall is up to 3.5 m in height and to our knowledge it has not been engineered. We recommend that the retaining wall be removed. If it is intended to keep the wall in place detailed investigation and analysis would be required and remedial work may be necessary.

#### 6.0 DESIGN RECOMMENDATIONS

#### **6.1 SITE PREPARATION**

#### 6.1.1 Stripping

Site stripping should be completed beneath foundations, retaining walls, pavement sections, and hard landscaping. Site stripping includes removal of any recently placed fill, organics, topsoil, weathered glacial till, and any other material considered to compromise the design recommendations stated herein to expose the underlying dense glacial till. We recommend that all foundations be lowered, if necessary, so that they are supported on a level subgrade of native silty sand and gravel glacial till.

SFA should be contacted to review stripped subgrade prior to placement of formwork.

The recently placed sand and gravel fill appears suitable for re-use as structural backfill which could be placed and compacted beneath grade supported slabs and for general backfill around the buildings. During stripping the sand and gravel fill should be carefully separated from the underlying topsoil and weathered glacial till.

#### 6.1.2 Engineered Fill

Any grade reinstatement beneath foundations, grade supported slabs, or pavement sections should be completed with "engineered fill". In the context of this report any "engineered fill" is defined as clean sand to sand and gravel fill, containing less than 8% fines, compacted in lifts to a minimum standard of 95% of its Modified Proctor Maximum Dry Density (ASTM D698) while at a moisture content that is within 2% of its optimum for compaction.

All fill materials should be placed and compacted under the review of SFA.

#### 6.1.3 Excavations

We anticipate that the excavation could be up to one and a half levels on the northeast side, decreasing in depth towards the southwest. The final excavation cut height should be determined once a site survey and proposed foundation grades are available.

Much of the excavation will likely be in dense glacial till. It is assumed that excavations would be sloped.

All excavations and trenching must conform to WorkSafeBC requirements or a professional engineer must review any excavations exceeding 1.2 m in depth prior to worker entry.

#### 6.2 Foundations

#### **6.2.1 Spread Foundations**

It is expected that foundations will be supported on the native subgrade soils of glacial till. Following the recommended site preparation, the subgrade soils are considered suitable to support conventional spread foundations at a serviceability limit state (SLS) bearing pressure of up to 175 kPa and a factored ultimate limit state (ULS) of 350 kPa.

All foundation subgrades must be reviewed by SFA prior to foundation construction.

#### 6.2.2 Settlement of Foundations

Post construction settlements are estimated to be less than 25 mm with differential settlements of less than 1 in 300.

#### 6.2.3 Seismic Design of Foundations

We expect the subgrade conditions underlying the site to be classified as Site Class C as defined in Table 4.1.8.4A of the 2018 British Columbia Building Code (2018 BCBC).

The subsurface soils beyond the depth of foundations are <u>not</u> considered prone to ground liquefaction or other forms of ground softening caused by earthquake induced ground motions.

#### 6.2.4 Frost Protection

All foundations should be located a minimum of 0.6 m below site grades for frost protection.

#### 6.3 Concrete Slabs on Grade

All grade supported concrete slabs, should be underlain by a minimum of 150 mm of 19 mm clear crushed gravel, to help prevent moisture from accumulating below the slab, placed over compacted "engineered fill" as described in this report. The gravel should be compacted in place. We recommend that a poly moisture barrier be placed overlying the gravel beneath the grade supported slabs to help reduce moisture within the concrete.

#### **6.4 Foundation Drainage**

We recommend that the building design include a conventional perimeter drainage system to help intercept and water at foundation-level and to ensure that groundwater does not accumulate below the floor slabs or adjacent to foundation walls. The under-slab fill should have a hydraulic connection to the perimeter drain to help ensure water does not build up below the slab or adjacent to foundation walls. This can be achieved with weep holes or by placing gravel below foundations.

#### 6.5 Backfill

Backfill adjacent to the foundations should be completed with free draining material such as clean sand and gravel or crushed rock fill containing less than 5% fines. The backfill should be compacted in lifts. In areas where the backfill will support hard landscaping or pavement areas the material should be compacted to a minimum of 95% of its Modified Proctor Maximum Dry Density while at a moisture content that is within 2% of its optimum for compaction.

#### 6.6 Earth Pressures on Buried Walls

We recommend that buried walls be designed for static and seismic earth pressures. We recommend that the wall be designed for a static pressure distribution of 5.0H (kPa) triangular, where H is the height of the restrained soil in metres. Dynamic loading induced by the design earthquake should be added to the static loads and should be taken as 1.7H (kPa) inverted triangular. The preceding loading recommendations assume that the backfill is a clean, free draining sand and gravel, the backfill is level behind the wall, and the wall is frictionless.

Our calculations assume that a back-of-wall drainage system will be installed to prevent the build up of any water pressure behind the walls. All earth pressures provided herein are based on unfactored soil parameters and are therefore unfactored loads.

#### 6.0 FIELD REVIEWS

As is normally required for municipal Letters of Assurance, SFA Geotechnical Inc. will carry out sufficient field reviews during construction to ensure that the geotechnical design recommendations contained within this report have been adequately communicated to the design team and to the contractors implementing the design. These field reviews are not carried out for the benefit of the contractors and therefore do not in any way effect the contractors' obligations to perform under the terms of his/her contract.

It is the contractors' responsibility to advise SFA Geotechnical Inc. (a minimum of 24 hours in advance) that a field review is required. Geotechnical field reviews are normally required at the time of the following:

1. Site Stripping	Review of excavation and stripped subgrade
2. Subgrade	Review of foundation subgrade
3. Backfill / Frost Depth	Review of adequacy of backfill and frost protection
4. Slab-on-grade	Review of subgrade preparation for any grade supported concrete slabs

It is critical that these reviews are carried out to ensure that our intentions have been adequately communicated. It is also critical that contractors working on the site view this document in advance of any work being carried out so that they are familiar with the sensitive aspects of the project. It is the responsibility of the developer to notify SFA Geotechnical Inc. when conditions or situations not outlined within this document are encountered.

Additional field reviews will be required for the proposed retaining walls. Once site grading is available and retaining wall designs are complete the number of additional field reviews required can be estimated.

#### 7.0 CLOSURE

This report is prepared solely for use by our client and their design team for this project as described to the general standards of similar work for similar projects in this area and no other warranty of any kind is expressed or implied. SFA Geotechnical Inc. accepts no responsibility for any other use of this report.

We are pleased to assist you with this project, and we trust this information is helpful and sufficient for your purposes at this time. However, please do not hesitate to call the undersigned if you should require any clarification or additional details.

For: SFA Geotechnical Inc.



Jaret Bull, M.A.Sc. Project Manager

Reviewed by: -5/2021

Steven Fofonoff, M.Eng., P.Eng. Principal



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# **BYLAW COMPLIANT WALLS**

THIS DRAWING IS FOR REFRENCE ONLY REFRENCE WALLS SHOWN IN A 1H:1V SLOPE WITH MAXIMUM EXPOSED FACE OF 4' AS PER BYLAW. COMPLIANT WALLS ALTHOUGH PROVIDE MORE USABLE SPACE, DO NOT CONFORM TO THE GRADES OR FIT NATURAL CONTOURS AS SUGGESTED IN THE HILLSIDE DESIGN GUIDELINES . PROPOSED WALLS FIT THE LANDSCAPE AND CONTOURS IN A MORE NATURAL AND ESTHETIC FORM.

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![](_page_37_Picture_4.jpeg)

#### 7.21 Retaining Walls

(a) In a residential zone, a single retaining wall shall:

- i. Not exceed a Height of 1.2 m measured from the average natural grade level at its base; and
- ii. Not be located within 0.6 m, measured horizontally, of any other retaining wall.

#### PART 22: RETAINING STRUCTURES

22.1. A registered professional shall undertake the design and conduct field reviews of the construction of a retaining wall greater than 1.2 meters in height.

22.2. Sealed copies of the design plan and *field review* reports prepared by the *registered* professional for all retaining walls greater than 1.2 meters in height shall be submitted to the Chief Building Official prior to acceptance of the works.

SEE GEOTECHNICAL DRAWINGS FOR SPECIFICATIONS MAXIMUM EXPOSED WALL HEIGHT OF 6'6" (2M) FACE

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